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Äspö Hard Rock Laboratory

Canister Retrieval Test

Sensors data report (Period 001026-030501) Report No:6

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May 2003

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This report concerns a study which was conducted for SKB. The conclusions and viewpoints presented in the report are those of the author(s) and do not necessarily coincide with those of the client.

Abstract

This report presents data from the measurements in the Canister Retrieval Test from 001026 to 030501.

The following measurements are made in the bentonite: Temperature is measured in 32 points, total pressure in 27 points, pore water pressure in 14 points and relative humidity in 55 points. Temperature is also measured by all relative humidity gauges. The positions of the measuring points in the bentonite are related to a coordinate system in the deposition hole.

The following measurements are made in the rock: Temperature is measured in 40 points, stresses are measured in 8 points and strain is measured in 9 points. Stresses and strains are also measured in the rock around the empty deposition hole located 6 m south of the test hole.

The following measurements are made in the canister: Temperature is measured every meter along two fiber optic cables and strain is measured in 76 points on the surface of the copper envelop. Temperature is measured in the steel insert in 18 points.

The following measurements are made on the plug: Force is measured in 3 of the 9 anchors and vertical displacement is measured in three points.

The water inflow to the filter mats on the rock surface is also measured.

The general conclusion is that the measuring systems and transducers seem to work well but the following problems have been noted: The strain measurements in the canister are not reported due to question marks regarding the relevance of the results. Most Vaisala relative humidity transducers have failed due to water saturation. All six Kulite total pressure transducers seem to yield erroneous results.

The wetting has continued during and partly been accelerated due to the applied high water pressure in the mats and the relative humidity sensors indicate that the bentonite between the rock and the canister is close to water saturation although the wetting seems to be somewhat uneven and the total pressure has not reached the expected values yet. Some clogging of the filters may explain the inhomogeneous appearance.

Sammanfattning

I denna rapport presenteras data från mätningar i Återtag under perioden 001026-030501.

Följande mätningar görs i bentoniten: Temperaturen mäts i 32 punkter, totaltryck i 27 punkter, porvattentryck i 14 punkter och relativa fuktigheten i 55 punkter. Temperaturen mäts även i alla relativa fuktighetsmätare. Varje mätpunkt relateras till ett koordinatsystem i deponeringshålet.

Följande mätningar görs i berget: Temperaturen mäts i 40 punkter, bergspänningar mäts i 8 punkter och töjningar i 9 punkter. Bergspänningar och töjningar mäts också i berget runt det tomma deponerinshålet 6 m söder om försökshålet.

Följande mätningar görs på ytan i kapselns kopparhölje: Temperaturen mäts varje meter längs två fiberoptiska kablar och töjning mäts i 76 punkter. Temperaturen mäts i stålinsatsen i kapseln i 18 punkter.

Följande mätningar görs på pluggen: Kraften mäts i 3 av de 9 stagen och vertikala förskjutningen mäts i tre punkter.

Vatteninflödet till filtermattorna mäts också.

En generell slutsats är att mätsystemen och givarna tycks fungera bra, men följande problem har noterats: Töjningsmätningarna i kapseln har inte redovisats p. g. a frågetecken beträffande mätresultatens relevans. De flesta av Vaisalas relativa fuktighetsmätare, har slutat fungera p. g. a. hög vattenmättnadsgrad. Alla sex Kulite totaltrycksgivare tycks ge felaktiga resultat idag.

Bevätningen har fortsatt och delvis accelererat pga det pålagda vattentrycket i mattorna och fuktighetsmätarna indikerar att bentoniten mellan berget och kapseln är nära vattenmättnad, fastän bevätningen tycks vara något ojämn och totaltrycket ännu inte har nått de förväntade värdena. En viss igensättning av filterna kan förklara det inhomogena uppförandet.

Contents

1	Introduction	9					
1 2.1 2.2 2.3 2.4 2.5 2.6 2.7 2.8 2.9	Introduction Comments General Total pressure, Geocon (App. A pages 33-36) Total Pressure, Kulite (App. A pages 37) Suction, Wescore Psychrometers (App. A pages 38-47) Relative humidity, Vaisala (App. A pages 48-52) Pore water pressure, Geocon (App. A pages 53-54) Pore water pressure, Kulite (App. A page 55) Water flow into the filters (App. A page 56) Encress on the plug (App. A page 57)	<pre>9 11 11 12 12 12 12 12 13 13 13 13 13</pre>					
2.9 2.10 2.11 2.12 2.13 2.14 2.15 2.16 2.17	 Displacement of the plug (App. A page 57) Displacement of the plug (App. A page 58) Canister power (App. A page 59) Temperature in the buffer (App. A pages 60-64) Temperature in the rock (App. A pages 65-68) Temperature on the canister surface, Optical fiber cables (App. A pages 69-70) Temperature inside the canister (App. A pages 71-73) Strain in the canister Rock stresses and strains 	13 13 14 14 14 14 14 14 14					
3	Geometry	15					
4 4.1 4.2 4.3 4.4 4.5 4.6 P of	Location of instruments Brief description of the instruments Measurements of temperature Measurement of total pressure in the buffer Measurement of pore water pressure in the buffer Measurement of the water saturation process Measurements of strain in the Canister Measurements of stresses and strain in the rock Measurements of forces on the plug Measurements of plug displacement Measurement of water flow into the permeable mats Strategy for describing the position of each device Position of each instrument in the bentonite Instruments in the rock Temperature measurements Stress and strain measurements Instruments in the canister Instruments at the plug	17 17 17 17 17 17 18 18 18 18 18 18 18 18 18 18 18 19 24 24 25 25 28 28					
Ref	erences	29					
App	Appendix A 31						
App	oendix B	75					

1 Introduction

The installation of the Canister Retieval Test was made during autumn 2000. In general the data in this report are presented in diagrams covering the time period 2000-10-26 to 2003-05-01. The time axis in the diagrams represents days from 2000-10-26. The diagrams are attached in Appendix A. The stress and strain measurements in the rock are reported separately by BBK. That report is attached as Appendix B.

A test overview with the positions of the measuring points and a brief description of the instruments is also presented in this report (chapters 3 and 4).

General comments concerning the collected data are given in chapter 2.

2 Comments

2.1 General

In this chapter short comments on general trends in the measurements are given. Sensors that are not delivering reliable data or no data at all are noted and comments on the data in general are given.

The slot between rock and bentonite block was filled with bentonite pellets and water at 001026. This date is also marked as start date. 1 m water head in the water supply tank was connected to the filters on 001102.

The heating of the canister started with an initially applied constant power of 700 W at 001027 that is one day after test start. The power was raised to 1700 W on 001113. The power was further raised to 2600 W on 010213.

At the end of 2001 two of the 36 electrical heaters failed due to short circuit to earth and no power was generated during one day between November 5 and 6 2001 (day 375). The heaters were also shut off during one week between March 4 and 11 2002 (days 495 to 502) for control measurements. The water pressure in the mats was stepwise increased to 800 kPa in the period 5/9 - 10/10 2002 (days 687-713). The power of the heaters in the canister was reduced to 2100 W on day 683 (10/9 -02)

The following events, which are important for understanding the results, have occurred this reporting half-year period:

- Day 749 (14/11 02): The mats were back flushed.
- Day 770 (5/12 -02): The water pressure in the mats on the rock wall was reduced from 800 kPa to 100 kPa.
- Day 805 (9/1 –03): The water pressure in the mats on the rock wall was increased to 400 kPa.
- Day 819 (23/1 –03): The water pressure in the mats on the rock wall was increased to 800 kPa.

The fifth report covered the period up to 021101. This report is the sixth one and covers the results up to 030501.

2.2 Total pressure, Geocon (App. A pages 33-36)

The increase in water pressure in the mats has resulted in an increased rate of total pressure increase for most transducers.

The measured pressure range is from 0 to 6.8 MPa. The highest pressure is indicated in the periphery of the bottom block (C1) and the center ring (R5), where all transducers yield a pressure between 3.1 and 6.8 MPa.

Sensor P104 was not installed. U106 was originally intended to be a pore pressure sensor but was replaced by a total pressure sensor. One of 21 sensors is out of order.

2.3 Total Pressure, Kulite (App. A page 37)

Six Kulite total pressure transducers are installed in the bentonite blocks. Unfortunately they don't seem to work properly. The reason for the malfunction is probably brakeage of the transducer connections at high pressures.

One sensor (P221) did not work from start 3 stopped working earlier and the two remaining transducers are having problems and may be out of order (under investigation).

2.4 Suction, Wescore Psychrometers (App. A pages 38-47)

Wescor psychrometers are only working at suction below 5000 kPa, which correspond to high relative humidity (above about 96%). 19 out of 26 transducer yield values that can be evaluated and have thus a high relative humidity. The interpretation of the values should be done with care since the evaluation is done with an automatic technique and the plateau required for proper evaluation not always formed. This explains why most transducers start their appearance from very low value. These low values are not correct but only an indication of that the relative humidity or suction is getting close to the measuring range. All diagrams are included (also diagrams with no evaluated values).

In Ring 5 eight out of nine transducers are indicating a high relative humidity (measurable suction), which confirm the total pressure measurements. In ring 10 all eight transducers indicate a high relative humidity.

2 out of 26 sensors are out of order.

2.5 Relative humidity, Vaisala (App. A pages 48-52)

App. A pages 37-41 show relative humidity and temperature measured with Vaisala transducers. The relative humidity results and the temperature results in ring 10 have been split into two diagrams (pages 52 and 53) since the data were interfering with each other.

All transducers in Cyl.1, Ring5 and Ring10 have stopped to work.

The reason for the successive malfunction of these is not clear but the transducers do not work very well at high relative humidity. It is interesting to note that the drying indicated by transducers W134 and W135, which are placed just above the canister, has stopped and turned into a slow wetting. Unfortunately, also these transducers have now

stopped working. All transducers between the rock and the canister in rings 5 and 10 indicate a high degree of saturation, which confirm the results of the Wescor psychrometers.

Only about 5 out of 25 sensors seem to still work properly. The major reason for malfunction is high degree of saturation. The reason for malfunction of some of the other transducers is under investigation.

2.6 Pore water pressure, Geocon (App. A pages 53-54)

U105, U107, and U108 have started yielding increasing water pressure to between 650 and 950 kPa, the reason being the increased water pressure in the mats. They are placed in Ring 5. The remaining sensors of this type show low pressure (0-150 kPa).

U106 is replaced by a total pressure sensor. All 11 sensors are working

2.7 Pore water pressure, Kulite (App. A page 55)

There are only one sensor of this type in Ring 10 and one in Cylinder 4. None of them have shown any increase in pore pressure.

2.8 Water flow into the filters (App. A page 56)

Measurement of water inflow into the filters started at 001102. The total inflow to the filters has since that date been 425 liter.

The inflow rate has strongly increased after start pressurizing the water in the mats at 020905 (day 678). The inflow is at present about 0.40 l/day.

2.9 Forces on the plug (App. A page 57)

The forces on the plug have been measured since 001106. The total force is about 6600 kN at 030501.

During the first about 50 days the plug was only fixed with 3 rods. When the total force exceeded 1500 kN the rest of the 9 rods were fixed in a prescribed manner. This procedure took place 12-14 December 2000 that is 46-48 days after test start. From that time only every third anchor is measured and the results should thus be multiplied with 3. The diagram shows both the actual measurements and after multiplication with 3.

2.10 Displacement of the plug (App. A page 58)

Problems, resulting in lack of data for some periods, have been overcome and the measurements are now working properly.

2.11 Canister power (App. A page 59)

The power 2100 W has been kept constant since the reduction.

2.12 Temperature in the buffer (App. A pages 60-64)

After reduction of power at 020909 (day 683) from 2600 W to 2100 W the highest measured temperature near the canister in Ring 5 has decreased from 86 °C to 75 °C.

The highest temperature gradient is about 0.72 degrees/cm (ring 5).

1 out of 32 sensors is out of order.

2.13 Temperature in the rock (App. A pages 65-68)

The maximum temperature measured in the rock (57 degrees) is measured in the central section on the surface of the deposition hole. Almost complete axial symmetry of the temperature measured in the rock is observed. A small increase in temperature in direction A (north) is caused be the neighboring experiment TBT that started its heating at 030326.

1 out of 40 sensors is out of order.

2.14 Temperature on the canister surface, Optical fiber cables (App. A pages 69-70)

The first diagram shows the maximum temperature plotted as a function of time. The maximum temperature measured on the canister surface is presently between 82 and 84 °C. The second diagram shows the distribution of the temperature along the cables at 030430. The length of the cable on the canister surface is only about 20 m and close to the entrances the lower surrounding temperatures have an influence on the measured temperature.

2.15 Temperature inside the canister (App. A pages 71-73)

After reduction of the power with 500 W, the max temperature inside the canister has been reduced from 107 °C to 94 °C.

Nine of these sensors have stopped working. All these sensors are placed near canister surface.

2.16 Strain in the canister

Continuous measurements have been made but so far no results have been produced due to evaluation problems.

2.17 Rock stresses and strains

Rock stresses and strains are reported in Appendix B.

3 Geometry

The test installation consists of a full-scale deposition hole, a copper canister equipped with electrical heaters and bentonite blocks (cylindrical and ring shaped). A plug of concrete and steel is anchored to the rock on top of the bentonite.

The saturation of the bentonite is attained artificially by vertical filter stripes. 16 stripes with a width of 0.1 meters and a length of 5.5 meters are applied on the surrounding rock.

Measurements are made in four vertical sections A, B, C and D according to Figure 3-1. Direction A-B is parallel to the tunnels axial with A headed almost against north.



Figure 3-1. Figure describing the instrument planes (A-D) and the coordinate system used when describing the instrument positions.

4 Location of instruments

4.1 Brief description of the instruments

The different instruments that are used in the experiment are briefly described in this chapter.

Measurements of temperature

Buffer

Thermocouples from BICC have been installed for measuring temperature in the buffer. Measurements are done in 32 points in the test hole. In addition, temperature gauges are built in into the capacitive relative humidity sensors (29 sensors) as well as in the pressure gauges of vibrating wire type (13 gauges). Temperature is also measured in the psychrometers.

Canister

Temperature is measured inside the canister (on the insert) in 19 points with PT-100 gauges. In addition temperature is measured on the surface of the canister with optical fiber cables. An optical measuring system called FTR (Fiber Temperature Laser Radar) from BICC is used.

Rock

Temperature in the rock and on the rock surface of the hole is measured in 40 points with thermocouples from BICC.

Measurement of total pressure in the buffer

Total pressure is the sum of the swelling pressure and the pore water pressure. It is measured with the following instrument types:

- Geocon total pressure cells with vibrating wire transducers. 15 cells of this type have been installed.
- Kulite total pressure cells with piezo resistive transducers. 6 cells of this type have been installed.

Measurement of pore water pressure in the buffer

Pore water pressure is measured with the following instrument types:

- Geocon pore pressure cells with vibrating wire transducer. 13 cells of this type have been installed.
- Kulite pore pressure cells with piezo resistive transducer. 2 cells of this type have been installed.

Measurement of the water saturation process

The water saturation process is recorded by measuring the relative humidity in the pore system, which can be converted into water ratio or total suction (negative water pressure). The following techniques and devices are used:

- Vaisala relative humidity sensor of capacitive type. 29 cells of this type have been installed. The measuring range is 0-100 % RH.
- Wescor psychrometers model PST-55. The devices measure the relative humidity in the pore system. The measuring range is 95.5-99.6 % RH corresponding to the pore water pressure -0.5 to -6MPa. 26 cells of this type have been installed.

Measurements of strain in the Canister

These measurements are not reported.

Measurements of stresses and strain in the rock

These measurements are not reported.

Measurements of forces on the plug

The force on the plug caused by the swelling pressure of the bentonite is measured in 3 of the 9 anchors. The force transducers are of the type GLÖTZL.

Measurements of plug displacement

Due to straining of the anchors the swelling pressure of the bentonite will cause not only a force on the plug but also displacement of the plug. The displacement is measured in three points with transducers of the type LVDT with the range 0 - 50 mm.

Measurement of water flow into the permeable mats

Water is supplied to the bentonite with filter strips attached to the rock surface. The water flow into these mats is measured by measuring the water volume in the supply tank with a differential pressure transmitter that measures the difference in pressure between the nitrogen in the top of the tank and the water in the bottom of the tank.

4.2 Strategy for describing the position of each device

Every instrument is named with a short unique name consisting of 1-2 letters describing the type of measurement and 3 figures numbering the device. Every instrument position in the buffer and rock is described with three coordinates according to Figure 3-1.

The r-coordinate is the horizontal distance from the center of the hole and the zcoordinate is the height from the bottom of the hole (the block height is set to 500 mm). The α -coordinate is the angle from the vertical direction B (almost south).

The short description of the positions in the diagrams differs between the buffer and the rock.

Buffer: Three positions with the following meaning: (bentonite block or cylinder number counted from the bottom $\$ direction A, B, C, or D $\$ radius in mm from center line)

Rock: Three positions with the following meaning: (distance in meters from the bottom $\land \alpha$ according to Fig 3-1 \land distance in meters from the hole surface)

The bentonite blocks are called cylinders and rings. The cylinders are numbered C1-C4 and the rings R1-R10 respectively (Figure 4-1).

4.3 Position of each instrument in the bentonite

Measurements are done in four vertical sections A, B, C and D according to Figure 3-1. Direction A and B are placed in the tunnels axial direction.

An overview of the positions of the instruments is shown in Fig 4-1. Exact positions are described in Tables 4-1 to 4-4.

The instruments are located in two main levels in the blocks, 50 mm and 160 mm, from the upper surface. The thermocouples have mostly placed in the 50mm level and the other gauges in the 160 mm level.



Figure 4-1 Schematic view over the instruments in four vertical sections and the block designation.

		Instrum	ent position	in block		Cable pos.		
Type and number	Block	Direction	α	r	Z	α	Fabricate	Remark
T101	Cyl. 1	Center	90	50	50	242	BICC	
T102	Cyl. 1	Center	90	50	250	238	BICC	
T103	Cyl. 1	Center	90	50	450	230	BICC	
T104	Cyl. 1	A	180	635	450	206	BICC	
T105	Cyl. 1	A	180	735	450	202	BICC	
T106	Cyl. 1	В	365	685	450	38	BICC	
T107	Cyl. 1	С	275	685	450	274	BICC	
T108	Cyl. 1	D	90	585	450	96	BICC	
T109	Cyl. 1	D	90	685	450	94	BICC	
T110	Cyl. 1	D	90	785	450	92	BICC	
T111	Ring 5	A	180	635	2950	224	BICC	
T112	Ring 5	A	180	735	2950	218	BICC	
T113	Ring 5	В	360	610	2950	318	BICC	
T114	Ring 5	В	360	685	2950	322	BICC	
T115	Ring 5	В	360	735	2950	324	BICC	
T116	Ring 5	С	270	610	2950	258	BICC	
T117	Ring 5	С	270	685	2950	260	BICC	
T118	Ring 5	С	270	735	2950	262	BICC	
T119	Ring 5	D	90	585	2950	44	BICC	
T120	Ring 5	D	90	635	2950	46	BICC	
T121	Ring 5	D	90	685	2950	48	BICC	
T122	Ring 5	D	90	735	2950	50	BICC	
T123	Ring 5	D	90	785	2950	52	BICC	
T124	Ring 10	A	180	635	5450	200	BICC	
T125	Ring 10	A	180	735	5450	194	BICC	
T126	Ring 10	D	90	585	5450	54	BICC	
T127	Ring 10	D	90	685	5450	56	BICC	
T128	Ring 10	D	90	785	5450	58	BICC	
T129	Cyl. 3	A	180	785	6250	166	BICC	
T130	Cyl. 3	В	365	585	6250	358	BICC	
T131	Cyl. 3	С	275	585	6250	280	BICC	
T132	Cyl. 4	Α	180	785	6950	66	BICC	

Table 4-1 Numbering and position of instruments for measuring temperature (T)

		Instrum	ent position	in block		Cable pos.		
Type and number	Block	Direction	α	r	Z	α	Fabricate	Remark
P101	Cyl. 1	Center	180	50	50	244	Geocon	
P102	Cyl. 1	Center	180	50	250	232	Geocon	
P103	Cyl. 1	A	185	585	250	208	Geocon	
P104	Cyl. 1	A	185	685	250	204	Geocon	
P105	Cyl. 1	A	185	785	250	186	Geocon	
P106	Cyl. 1	В	365	585	250	40	Geocon	
P107	Cyl. 1	В	365	785	250	2	Geocon	
P108	Cyl. 1	С	275	585	250	278	Geocon	
P109	Cyl. 1	С	275	785	250	270	Geocon	
P110	Ring 5	A	185	585	2750	228	Geocon	
P111	Ring 5	А	185	685	2750	222	Geocon	
P112	Ring 5	A	185	785	2750	188	Geocon	
P113	Ring 5	В	365	535	2750	36	Geocon	
P114	Ring 5	В	365	825	2750	16	Geocon	
P115	Ring 5	С	275	585	2750	296	Geocon	
P116	Ring 5	С	275	785	2750	290	Geocon	
P117	Ring 10	Center	180	50	5250	24	Kulite	
P118	Ring 10	A	180	585	5250	216	Geocon	
P119	Ring 10	А	180	685	5250	198	Geocon	
P120	Ring 10	A	180	785	5250	192	Geocon	
P121	Ring 10	В	365	585	5250	20	Kulite	
P122	Ring 10	В	365	785	5250	18	Kulite	
P123	Ring 10	С	275	585	5250	286	Kulite	
P124	Ring 10	С	275	785	5250	284	Kulite	
P125	Cyl. 3	Center	180	50	6250	158	Geocon	
P126	Cyl. 3	A	180	585	6250	162	Geocon	
P127	Cyl. 4	Center	180	50	6750	64	Kulite	

 Table 4-2
 Numbering and position of instruments for measuring total pressure (P)

Table 4-3 Numbering and position of instruments for measuring pore water pressure (U)

		Instrum	Instrument position in block			Cable pos.		
Type and number	Block	Direction	α	r	Z	α	Fabricate	Remark
U101	Cyl. 1	Center	270	50	50	246	Geocon	
U102	Cyl. 1	Center	270	50	250	236	Geocon	Horizontal
U103	Cyl. 1	A	175	585	250	126	Geocon	
U104	Cyl. 1	A	175	785	250	178	Geocon	
U105	Ring 5	A	175	585	2750	138	Geocon	
U106	Ring 5	A	175	785	2750	180	Geocon	
U107	Ring 5	В	355	535	2750	314	Geocon	
U108	Ring 5	В	355	825	2750	348	Geocon	
U109	Ring 5	С	265	585	2750	256	Geocon	In the slot
U110	Ring 5	С	265	825	2750	264	Geocon	In the slot
U111	Ring 10	A	175	585	5250	146	Geocon	
U112	Ring 10	A	175	785	5250	152	Geocon	
U113	Cyl. 3	Center	270	50	6250	156	Geocon	
U114	Cyl. 4	Center	270	50	6950	62	Kulite	

		Instrument position in block		Cable pos.				
Type and number	Block	Direction	α	r	Z	α	Fabricate	Remark
W101	Cvl. 1	Center	360	50	50	248	Vaisala	
W102	Cyl. 1	Center	360	400	160	240	Vaisala	
W103	Cyl. 1	Center	360	50	450	234	Vaisala	Horizontal
W104	Cyl. 1	А	180	585	340	128	Vaisala	
W105	Cyl. 1	Α	180	685	340	132	Vaisala	
W106	Cyl. 1	A	180	785	340	184	Vaisala	
W107	Cyl. 1	Α	170	585	340	124	Wescor	
W108	Cyl. 1	A	170	685	340	130	Wescor	
W109	Cyl. 1	A	170	785	340	134	Wescor	
W110	Cyl. 1	В	360	585	340	304	Vaisala	
W111	Cyl. 1	В	360	785	340	360	Vaisala	
W112	Cyl. 1	В	360	685	340	308	Vaisala	
W113	Cyl. 1	В	355	585	340	302	Wescor	
W114	Cyl. 1	В	355	685	340	306	Wescor	
W115	Cyl. 1	В	355	785	340	310	Wescor	
W116	Cyl. 1	С	270	585	340	250	Wescor	
W117	Cyl. 1	С	270	685	340	252	Wescor	
W118	Cyl. 1	С	270	785	340	254	Vaisala	
W119	Ring 5	A	180	585	2840	226	Vaisala	
W120	Ring 5	A	180	685	2840	220	Vaisala	
W121	Ring 5	A	180	785	2840	182	Vaisala	
W122	Ring 5	A	170	585	2840	136	Wescor	
W123	Ring 5	A	170	685	2840	140	Wescor	
W124	Ring 5	A	170	785	2840	142	Wescor	
W125	Ring 5	В	360	535	2840	316	Vaisala	In the slot
W126	Ring 5	В	360	685	2840	34	Vaisala	
W127	Ring 5	В	360	785	2840	350	Vaisala	
W128	Ring 5	В	350	535	2840	312	Wescor	In the slot
W129	Ring 5	В	350	685	2840	320	Wescor	
W 130	Ring 5	В	350	785	2840	340	Wescor	In the elet
W 131	Ring 5		270	505	2040	294	Wescor	In the slot
W 132	Ring 5		275	795	2040	292	Wescor	
W 133	Ring 5 Ring 10	Center	360	785 50	2040 5340	200	Vaisala	
W134 W135	Ring 10		180	262	5340	22	Vaisala	
W135	Ring 10	A A	180	585	5340	20	Vaisala	
W130	Ring 10	Δ	180	685	5340	106	Vaisala	
W138	Ring 10	A	180	785	5340	190	Vaisala	
W139	Ring 10	A	170	585	5340	144	Wescor	
W140	Ring 10	A	170	685	5340	148	Wescor	
W141	Rina 10	A	170	785	5340	150	Wescor	
W142	Ring 10	В	360	585	5340	328	Vaisala	
W143	Rina 10	B	360	685	5340	332	Vaisala	
W144	Ring 10	В	360	785	5340	336	Vaisala	
W145	Ring 10	B	355	585	5340	326	Wescor	
W146	Ring 10	В	355	685	5340	330	Wescor	
W147	Ring 10	В	355	785	5340	334	Wescor	
W148	Ring 10	С	270	585	5340	266	Wescor	
W149	Ring 10	С	270	685	5340	268	Wescor	
W150	Ring 10	С	270	785	5340	272	Vaisala	
W151	Cyl. 3	Center	360	50	6250	154	Vaisala	
W152	Cyl. 3	Α	180	585	6250	160	Vaisala	
W153	Cyl. 3	В	360	585	6250	356	Vaisala	
W154	Cyl. 3	С	270	585	6250	276	Wescor	
W155	Cyl. 4	Center	360	50	6840	60	Vaisala	

Table 4-4 Numbering and position of instruments for measuring water content (W)

4.4 Instruments in the rock

Temperature measurements

40 thermocouples are placed in the rock and on the rock surface of the deposition hole. Holes have been bored in three directions on three levels and one additional hole has been bored in the bottom of the deposition hole i.e. totally 10 holes. They are led from the rock, over the gap between rock and bentonite and up along the bentonite block periphery. The position of the thermocouples in the rock is shown in Table 4-5.

				Cable pos.		
Type and number	Level	Direction	Distance from rock surface	α	Fabricate	Remark
TR101	0	Center	0.000	70°-90°	BICC	
TR102	0	Center	0.375	70°-90°	BICC	
TR103	0	Center	0.750	70°-90°	BICC	
TR104	0	Center	1.500	70°-90°	BICC	
TR105	0.61	10°	0.000	4°-14°	BICC	
TR106	0.61	10°	0.375	4°-14°	BICC	
TR107	0.61	10°	0.750	4°-14°	BICC	
TR108	0.61	10°	1.500	4°-14°	BICC	
TR109	0.61	80°	0.000	70°-90°	BICC	
TR110	0.61	80°	0.375	70°-90°	BICC	
TR111	0.61	80°	0.750	70°-90°	BICC	
TR112	0.61	80°	1.500	70°-90°	BICC	
TR113	0.61	170°	0.000	168°-176°	BICC	
TR114	0.61	170°	0.375	168°-176°	BICC	
TR115	0.61	170°	0.750	168°-176°	BICC	
TR116	0.61	170°	1.500	168°-176°	BICC	
TR117	3.01	10°	0.000	4°-14°	BICC	
TR118	3.01	10°	0.375	4°-14°	BICC	
TR119	3.01	10°	0.750	4°-14°	BICC	
TR120	3.01	10°	1.500	4°-14°	BICC	
TR121	3.01	80°	0.000	70°-90°	BICC	
TR122	3.01	80°	0.375	70°-90°	BICC	
TR123	3.01	80°	0.750	70°-90°	BICC	
TR124	3.01	80°	1.500	70°-90°	BICC	
TR125	3.01	170°	0.000	168°-176°	BICC	
TR126	3.01	170°	0.375	168°-176°	BICC	
TR127	3.01	170°	0.750	168°-176°	BICC	
TR128	3.01	170°	1.500	168°-176°	BICC	
TR129	5.41	10°	0.000	4°-14°	BICC	
TR130	5.41	10°	0.375	4°-14°	BICC	
TR131	5.41	10°	0.750	4°-14°	BICC	
TR132	5.41	10°	1.500	4°-14°	BICC	
TR133	5.41	80°	0.000	70°-90°	BICC	
TR134	5.41	80°	0.375	70°-90°	BICC	
TR135	5.41	80°	0.750	70°-90°	BICC	
TR136	5.41	80°	1.500	70°-90°	BICC	
TR137	5.41	170°	0.000	168°-176°	BICC	
TR138	5.41	170°	0.375	168°-176°	BICC	
TR139	5.41	170°	0.750	168°-176°	BICC	
TR140	5.41	170°	1.500	168°-176°	BICC	

 Table 4-5
 Numbering and positions of thermocouples in the rock

Stress and strain measurements

See Appendix B.

4.5 Instruments in the canister

The canister is instrumented with optical fiber cables on the copper surface, thermocouples in the steel insert and strain gauges on the inner and outer surface of the copper envelop in canister.

Optical fiber cables

Figure 4-2 shows how the two optical fiber cables are placed on the canister surface. Both ends of a cable are used for measurements. This means that the two cables are used as four measuring channels as described in Table 4-6.

With this laying the cable will enter and exit the surface at almost the same position. Curvatures are shaped as a quarter circle with a radius of 20 cm. The cable is placed in a milled out channel on the surface. The channel has a width and a depth of just above 2 mm

Channel 1	Outlet of cable 1
Channel 2	Inlet of cable 1
Channel 3	Outlet of cable 2
Channel 4	Inlet of cable 2

Table 4-6. Combination of cables and channels

Figure 4-3 shows the location of the thermocouples on the steel insert inside the canister.

Thermocouple, PT100

Temperature in the steel insert measured at 18 point of measuring with. thermocouple of type PT100. Figure 4-4 shows how these thermocouple are placed



Figure 4-2. Laying of two optical fibre cables with protection tube of Inconel 625 (outer diameter 2 mm) for measurement of the canister surface temperature (surface unfolded).



Figur 4-3. Location of thermocouples inside the canister

4.6 Instruments at the plug

Three force transducers and three displacement transducers have been placed on the plug to measure the force of the anchors and the displacement of the plug. The location of these transducers can be described in relation to Fig 4-4, which shows a schematic view of the plug with the slots, rods and cables.

The rods are numbered 1-9 anti-clockwise and number 1 is assumed to be the northern rod in direction A. The force transducers are placed on rods 3, 6, and 9. The displacement transducers are placed between the rods 5 cm from the rock surface of the hole and according to Table 4-6. They are fixed on the rock surface and measure thus the displacement relative the rock.



Figure 4-4. Schematic view of the deposition hole, showing the position of the slots, the rods and the cables from the canister.

Transducer No.	Located between rods No.
1	4 and 5
2	7 and 8
3	1 and 2

References

/1-1/ Sanden T, Börgesson L. Report on instrument positions and preparation of bentonite blocks for instruments and cables May 2000. SKB IPR-00-14



Measured data










Total pressure - Ring 10 (001026-030501) Geokon



Total pressure - Cylinder 3 (001026-030501) Geokon









Suction in the buffer - Cyl.1 (001026-030501) Wescor

Suction in the buffer - Cyl.1 (001026-030501) Wescor







Suction in the buffer - Ring 5 (001026-030501) Wescor







F



















Relative humidity - Ring 10 (001026-030501) Vaisala

























Canister power (001026-030501)

















Temperature in the rock - level 0,6 m (001026-030501)


















Temperature inside the canister (001026-030501) PT-100

72



Appendix B

<u>BBK</u>

STRESS AND STRAIN MEASUREMENT OF THE ROCK MASS RETRIEVAL TEST AT ÄSPÖ

Measuring period

2002-11-01 - 2003-04-30

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2003-05-26

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Со	ntents	Page
1	EXTENT	81
2 2.1 2.2 2.3	TECHNICAL BACKGROUND Vibrating wire embedment biaxial stressmeters Embedded strain meters Cement 83	83 83 83
3 3.1 3.2 3.3	FIELD MEASUREMENT Installation work Location of instruments Registration	85 85 85 85
4 4.1 4.2 4.3 4.4	COMPUTER PROCESSING OF FIELD DATA Evaluation of stresses 4.1.1 Radial deformations 4.1.2 Calculation of deformation to stresses Evaluation of strain 4.2.1 Calculation of strain Material parameters Processing	89 89 89 90 90 90 91
5 5.1 Resu	RESULTS Results of canister hole 1 5.1.1 Stress change for each biaxial stressmeter 5.1.2 Strainmeter A1:1, B1:1, C1:1, D1:1, D1:2 (temperature compensated) 5.1.3 Strainmeter C1:2 (temperature compensated) Its of canister hole 2 5.1.4 Stress change for each biaxial stressmeter 5.1.5 Strainmeter B2:1, C2:1, C2:2 (temperature compensated)	93 97 97 101 101 102 102 106

1 EXTENT

BBK AB and NCC Teknik have, on commission of SKB, ÄSPÖ hard rock laboratory, performed rock mechanical measurements in the Repository tunnel at Äspö. The measurement program comprises of registration of the stress and the strain response around the two canister holes during drilling and heating of the rock mass.

In the first phase, the response of the rock mass was monitored during the drilling of the two canister holes (QPTD F69-00-21). This second phase includes the response registered during a heating phase. The heating experiment started on 2000-10-27 and will continue for about five years.

The aim of the instrumentation is to monitor the stress changes due to heating of the rock mass in a canister hole. The strain meter is used to monitor the relative changes of strain of the intact rock and across fractures.

The commission extends over field measurement and evaluation.

BBK AB is responsible for measuring equipment, the mobilization, field measurement, the computer processing. BBK AB and NCC Teknik are responsible for the interpretation and report of the measurements.

This report presents the measurement results during the period of the heating phase from 2002-11-01 to 2003-04-30.

2 TECHNICAL BACKGROUND

2.1 Vibrating wire embedment biaxial stressmeters

The biaxial stressmeter, Geokon model 4350, is designed to measure compressive stress changes in rock, salt, concrete or ice. Principal stress changes are measured in the plane perpendicular to the borehole axes. The stressmeter consists of a high-strength steel cylinder that is grouted into a 60 mm borehole. Stress changes in the host material cause the cylinder to deform.

The radial deformation of the cylinder is measured by means of three pairs of vibrating wire sensors spaced at 60° intervals. Changes of stress produce corresponding changes in the resonant frequency of the sensors. These changes of frequency can be related to stress changes using factory-supplied calibrations. Longitudinal strain sensors and temperature sensors are also included in the stressmeter.

2.2 Embedded strain meters

The vibrating wire strain gages are designed for direct embedment in concrete. The strainmeter is 15 cm long and commonly used for strain measurement in foundations, piles, bridges, dams, tunnel liners etc.

The strain is measured using the vibrating wire principle. A length of a steel wire is tensioned between two end blocks that are embedded directly in concrete. Deformation (i.e. strain changes) of the concrete mass will cause the two end blocks to move relative to one another, thus altering the tension in the steel wire. The tension is measured by excitation of the wire and measuring its resonant frequency of vibration using an electro magnetic coil.

2.3 Cement

Special expansive grout was used to insure that the gage is in complete contact with the surrounding rock. The instruments are grouted in special cement from Denmark named Densitop T2. This cement is chosen to have as similar properties as the rock as possible. The compression strength is 150 MPa. The coefficient of expansion is approximate 8.5 microstrain/C° that is similar to hard rock as granite and as 85 % of common concrete.

3 FIELD MEASUREMENT

3.1 Installation work

Installation of the stressmeter gage is accomplished by inserting the gage into a groutfilled borehole using a setting tool and self-aligning setting rod.



The stress cell is orientated so that the first vibrating wire is orientated tangentially to the canister hole. The second string is orientated 60° from tangential direction and the third string is orientated 120° from tangential direction.

The strainmeters were fixed to a 6 mm glasfiber rod and pushed into the grout after the stress cell was installed.

3.2 Location of instruments

See figure 3.1 and figure 3.2.

3.3 Registration

A datalogger type Campbell CR10X has captured the measurements, which have been recorded once every hour during the period 2000-10-01 to 2001-01-01, and once every six hours during the period 2001-01-03 to 2003-04-30.



Figure 3.1 Measurement Layout



<mark>○ A1:1</mark> A1⊡

C1 O (1:1, C1:2

 $A \longrightarrow \overset{D1}{\underset{\substack{0:\\D1:1,\\D1;2}}{D1}}$

Canister Hole 2



B1

Bi ⊡O_{B1:1}



SECTION A-A



- □ Biaxial stressmeter
- Deformationmeter

Figure 3.2 Location of instruments

4 COMPUTER PROCESSING OF FIELD DATA

4.1 Evaluation of stresses

The stress changes are evaluated from the measured deformations registered by the vibrating wires.

4.1.1 Radial deformations

Radial deformation for each of the strings are calculated with the equation:

 $V_r = (R_1 - R_0)^*$ Gagefactor (in. or mm)

 V_r = Radial deformation for each of the strings

 R_1 = Deformation reading in digits (= frequency ² / 1000)

 R_0 = Deformation zero reading in digits (= frequency ² / 1000)

4.1.2 Calculation of deformation to stresses

The magnitude and the direction of the stress changes are determined from the measured radial deformation of the sensor in three directions.

The equations below give the magnitude and the direction of the maximum stress increase and reduction in a plane perpendicular to the borehole axes:

Maximal stress increase

$$p = \frac{1}{2} \left[\frac{1}{3B} \left(\left(2V_{r_1} - V_{r_2} - V_{r_3} \right)^2 + 3\left(V_{r_2} - V_{r_3} \right)^2 \right)^{\frac{1}{2}} + \frac{1}{3A} \left(V_{r_1} + V_{r_2} + V_{r_3} \right) \right]$$

 V_r = Radial deformation for string 1

 V_{r_2} = Radial deformation for string 2

 V_r = Radial deformation for string 3

A, B = Coefficients depending on the sensor geometry and the material properties

Maximal stress reduction

$$q = \left[\frac{1}{3A}\left(V_{r_1} + V_{r_2} + V_{r_3}\right) - p\right]$$

The angle of the maximal stress increase

The angle in the plane perpendicular to the borehole axes is measured clockwise from the tangential direction of the canister hole.

$$\theta = \frac{1}{2} \cos^{-1} \left[\frac{V_{r_1} - A(p+q)}{B(p-q)} \right]$$

4.2 Evaluation of strain

Nine strain gages were installed in the same boreholes as the biaxial stressmeters.

4.2.1 Calculation of strain

Strain measurement were calculated as temperature compensated load related strain with the following equation:

$$\mu_{true} = (R_1 - R_0) * B + (T_1 - T_0)(C_1 - C_2)$$

 μ_{true} = temperature compensated microstrain

 R_1 and R_0 =Digits reading

B = batch calibration factor

 C_1 and C_2 are the coefficients of expansion of steel and concrete, 12.2 microstrian/ C° and 8.5 microstrain/C°.

4.3 Material parameters

Material parameters used in the calculations are as the following:

- Young's modulus of intact rock 69 Gpa
- Poisson's ratio of intact rock 0.25
- Coefficients of expansion of steel 12.2 microstrain/ C°
- Coefficients of expansion of concrete 8.5 microstrain/C°

4.4 Processing

The raw data registered have been processed in Microsoft Excel software. The calculation and evaluation gathered from the deformations in the plane perpendicular to the borehole axes are presented as:

Temperature

Radial deformation of the three pairs of wires in plane perpendicular to borehole axes

Maximal stress increases and stress reduction in plane perpendicular to borehole axes

Orientation of maximal stress increase in plane perpendicular to borehole axes

Strain measurement

5 RESULTS

A summary of the results of the stress changes is presented in Table 1. These values represent total changes in stress magnitude and stress orientation during each reporting period during phase 2 (heating) together with maximum changes in stress magnitude and orientation recorded during phase 1 (drilling). Figure 5.1 presents graphically the maximum typical changes in stress magnitude and orientation during this reporting period from 2002-11-01 to 2003-04-30. These results are not compensated for temperature changes, nor are they compensated for longitudinal stress changes.

Although the measurements of stressmeters are affected by temperature, the temperature compensated results of stressmeters are not presented in this report because a further detailed study of the relationship between temperature and stressmeter measurements will be required in order to quantify these effects. In addition, the longitudinal stress changes are affected by temperature changes and for this reason these longitudinal measurements are not currently used on determination of radial stresses. Results of the biaxial stressmeter without temperature or longitudinal stress compensation, and the strainmeter with temperature compensation from each borehole for the period 2002-11-01 to 2003-04-30 are summarized on the diagrams in section 5.1 and 5.2.

The diagrams in section 5.1 and 5.2 for each borehole display:

- Maximum stress increases and stress reductions in plane perpendicular to borehole axes.
- Orientation of maximum stress increase in plane perpendicular to borehole axes.
- Strain measurements presented in the diagrams are temperature-compensated with the unit of microstrain. Positive values represent elongation.

The data during this reporting period indicate that:

• The temperature is inhomogeneous around these two canister holes.

Hole 1

Around canister hole 1, the temperatures increased quickly at the end of March from an average of about 25 up to about 50 to 56°C.

At three of the four stressmeters, the maximum stress increases have risen by about 13 MPa to a value of about 34MPa. At the fourth stressmeter, the maximum stress increase has risen by 18 MPa to a value of 45MPa.

Maximum stress reductions have shown less consistency between the gauges, and the calculated values of stress decrease were lower in magnitude than the stress increases. At two of the instruments the change was about 1Mpa, while another instrument recorded a decrease of 7MPa since the previous reporting period.

The orientation of the maximum stress increase was relatively unchanged during this reporting period with the exception of stressmeter A1 which showed a decrease of 5 degrees during the beginning of November 2002, five months before the increase in temperature occurred.

Measurements taken at the strain gauge locations display a consistent decrease in microstrain with increasing temperature. This indicates increasing compression along the length of the strainmeter. Four of the six strainmeters had a decrease of 200 microstrain, while the remaining two decreased by about 100 microstrain.

Hole 2

Around canister hole 2 the temperatures and instruments measurements have been relatively stable during this reporting period. Instruments located at greater depths (B2, C2 and B2:1) show temperatures on average 25°C higher than at the more shallow instruments.

The stressmeters around canister hole 2 show small variations in stresses (1to 4 MPa) which generally follow the recorded temperature changes.

The orientation of the stress field around stressmster A2 changed noticeably during April 2003, decreasing from approximately -15 degrees to about -9 degrees which means that the maximum stress increase was oriented more closely to tangential to the canister hole at the end of the reporting period. The orientation of the stresses around the other stressmeters changed less than 2 degrees.

At stressmeter C2 the recorded data from one of the two longitudinal gage as well as radial gage Vr5 have become -99999, which indicates that an error occurs while taking the readings. The vibrating wire gages are functioning properly, and can be read using a portable readout device; however the datalogger system is unable to record these measurements. The reason for this has yet to be determined. These individual readings result in an incorrect determination of stress changes and orientations.

Recorded changes in the strainmeters have been on the order of a maximum about 10 to 20 microstrain, and the measured changes in gage lengths have reflected the changes in temperature with an increase in temperature resulting in shortening of the gage length.



Figure 5.1 Maximum stress changes uncompensated for temperature

Stress-				Measuring perio	q				
meter	Phase 1: D	rilling phase				Phase 2: Heating p	hase		
				Period: 2000-10-	01 to 2001-11-	30	Period: 2000-10	-01 to 2001-11-	30
	Typical max.	Typical max.	Orientation of	Typical max.	Typical max.	Orientation of	Typical max.	Typical max.	Orientation of
	stress increase	stress	max. Stress	stress increase	stress	max. Stress	stress increase	stress	max. Stress
	(Mpa)	reduction	increase (degree)	(Mpa)	reduction	increase (degree)	(Mpa)	reduction	increase (degree)
		(Mpa)			(Mpa)			(Mpa)	
A1	17.0	-10.0	11.5	22.7	-10.9	10.3	22.6	-10.8	10.3
B1	15.0	-11.0	17.0	21.1	-10.3	16.8	26.9	-10.5	16.8
C1	12.0	-7.4	0	21.9	6.6-	5.4	21.9	-9.8	5.4
D1	22.0	-11.0	0	25.8	-9.1	0	26.3	-9.4	0
A2	5.0	-4.0	-11.0	4.4	-3.3	-18.3	3.7	-3.3	-17.7
B2	11.0	-7.3	-7.5	31.1	4.6	-14.1	57.9 (*)	-4.6	-21.3
C2	11.0	-7.7	29.5	29.1	-6.6	31.2	33.7 (*)	-7.6	30.2
D2	9.3	-7.2	38.5	16.5	6.6-	38.7	16.2	-9.9	38.3
Stress-	Measuring perio	pc		Measuring perio	þ				
meter	Phase 2: Heating	g phase con't	(see Note 5)	Phase 2: Heating	g phase con't				
	Period: 2002-05-	-01 to 2002-10	-31	Period: 2002-11-	01 to 2003-04-	30			
	Typical max.	Typical max.	Orientation of	Typical max.	Typical max.	Orientation of			
	stress increase	stress	max. Stress	stress increase	stress	max. Stress			
	(Mpa)	reduction	increase (degree)	(Mpa)	reduction	increase (degree)			
		(Mpa)			(Mpa)				
A1	23.3	-11.0	10.4	34.7(*)	-17	5.7			
B1	21.1	-10.6	15.9	34(*)	-9.6	17.1			
C1	22.3	-10.4	5.3	34(*)	-9.1	5.7			
D1	27.3	-9.7	-2.9	45(*)	-5.6	-4.3			
A2	4.3	-3.9	-13.9	3.5	-3.5	-9(*)			
B2	34 to 57	4 to 21	-0.6 to -8	32 to 54	6 to 22	-1 to -10			

Notes:

1. Positive values of stress indicate compression, negative values of stress indicate extension.

to -10 9 to 33

88

-0.6 to -8 22 to 33 ဂု

32

38.7

5.3 to 22. -10.1

16.0 50 to 7

3

Positive values of orientation indicate anti-clockwise rotation, negative values of orientation indicate clockwise rotation. с.

Typical maximum stress increases and reductions represent maximum stabilised readings during the measuring period. (*) indicates that the reading had not stabilised during the measuring preiod. . ຕ

Orienataion of maximum stress increase is the orientation that corresponds to the typical maximum stress increase. 4.

Calculation and processing methods revised during this reporting period to eliminate compensation for both temperature and longitudinal strains until further studies on these efftecs are carried out. ъ.

Table 1. Summary of results for stress measurements

5.1 Results of canister hole 1

5.1.1 Stress change for each biaxial stressmeter Biaxial stressmeter A1 uncompensated for temperature:



Biaxial stressmeter A1 Maxiumum stress increase and stress reduction in plane perpendicular to borehole axis

Hole 1

A1

Hole 2

Biaxial stressmeter A1 Orientation of maxiumum stress increase in plane perpendicular to borehole axis





Biaxial stressmeter B1 uncompensated for temperature:



Biaxial stressmeter B1 Maximum stress increase and stress reduction in plane perdendicular to borehole axis

Biaxial stressmeter B1 Orientation of maximum stress increase in plane perdendicular to borehole axis





Biaxial stressmeter C1 Orientation of maximum stress increase in plane perpendicular to borehole axis







Biaxial stressmeter D1 Maximum stress increase and stress reduction in plane perdendicular to borehole axis



Biaxial stressmeter D1 Orientation of maximum stress increase in plane perpendicular to borehole axis



5.1.2 Strainmeter A1:1, B1:1, C1:1, D1:1, D1:2 (temperature compensated)





Positive values of microstrain represent elongation

5.1.3 Strainmeter C1:2 (temperature compensated)



Positive values of microstrain represent elongation

Results of canister hole 2 5.1.4 Stress change for each biaxial stressmeter Biaxial stressmeter A2 uncompensated for temperature:





Biaxial stressmeter A2 Maximum stress increase and stress decrease in plane perdendicular to borehole axis

Biaxial stressmeter A2 Orientation of maximum stress increase in plane perpendicular to borehole axis





Biaxial stressmeter B2 uncompensated for temperature:

Hole 1

Hole 2

 $Biaxial \ stressmeter \ B2 \\ Orientation \ of \ maximum \ stress \ increase \ (\sigma_1) \ in \ plane \ perdendicular \ to \ borehole$



Biaxial stressmeter C2 uncompensated for temperature:





Biaxial stressmeter C2 Maximum stress increase and stress reduction in plane perpendicular to borehole axis

Biaxial stressmeter C2 Orientation of maximum stress increase in plane perpendicular to borehole axis



Biaxial stressmeter D2 uncompensated for temperature:





Biaxial stressmeter D2 Maximum stress increase and stress reduction in plane perpendicular to borehole axis

Biaxial stressmeter D2 Orientation of maximum stress increase in plane perpendicular to borehole axis



5.1.5 Strainmeter B2:1, C2:1, C2:2 (temperature compensated)



Positive values of microstrain represent elongation

